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# **Research on Initial Stiffness of Top-and-Seat Angle Semirigid Connections**

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**Abstract:** Based on the mechanical characteristics, deformation characteristics of top-and-seat angle semirigid connections, the initial stiffness of the connections were studied. The initial stiffness were derived theoretically with component method. Analytical formulas and fitted formulas were obtained, and these formulas were analysed comparatively with test data and FEA results. The results of the research indicated that the initial stiffness of top-and-seat angle semirigid connections were expressed accurately by the fitted formula in the paper, and fit accurately with test data and FEA results. The results of practical calculation method of such semirigid connections.

Keywords: Initial stiffness, semi-rigid connections, top-seat angle connections.

#### **1. INTRODUCTION**

The three parameters power model was shown in Eq. (1) below:

$$M = \frac{R_{\rm ki}\theta}{\left[1 + (\theta/\theta_0)^n\right]^{1/n}}$$
(1)

where,  $R_{\rm ki}$  is initial stiffness of the connections,  $\theta_0 = M_{\rm u}/R_{\rm ki}$  (a reference plastic rotation),  $M_{\rm u}$  is ultimate moment capacity and *n* is shape parameter.

# 2. THE INITIA STIFFNESS (THE COMPONENT METHOD)

When calculating initial stiffness, the members effecting initial stiffness were divided compressive region and tensile region according to different stress [1]. Tensile region: initial stiffness of tensile bolt  $R_{t,b}$ ; initial stiffness of bending column flange  $R_{t,cf}$ ; initial stiffness of tensile column web  $R_{t,cw}$ ; initial stiffness of bending angle  $R_{t,a}$ .Compressive region: initial stiffness of compressive column web  $R_{c,cw}$ .

#### 2.1. The Initial Stiffness of Tensile Bolt

According to Hooke's law, the displacement of single tensile bolt was shown in Eq. (2) and the initial stiffness was shown in Eq. (3):

$$\Delta_{\rm b,t} = \frac{F_{\rm b,t}l_{\rm b}}{EA_{\rm b}} \tag{2}$$

$$R_{t,b} = n_b \frac{EA_b}{l_b} \tag{3}$$

Where  $\Delta_{b,t}$  is displacement of tensile bolt;  $F_{b,t}$  is bolt tension;  $A_b$  is effective area of bolt;  $l_b$  is calculated length of bolt,  $l_b = t_{cf} + t_a$ ,  $t_a$  is angle thickness,  $t_{cf}$  is thickness of column flange;  $R_{t,b}$  is tensile stiffness of bolt;  $n_b$  is numbers of tensile bolt closed to beam flange [2].

#### 2.2. The Initial Stiffness of Compressive Column Web

The pressure on compressive flange of beam was transferred to column web by compressive angle, and pressure on column web was regarded as distributed load. The effective width  $b_{\text{eff,cw}}$  of compressive column web was shown in Fig. (2) in previous work by [3] The displacement and the initial stiffness of compressive column web were shown in Eq. (4) and Eq. (5) below:

$$\Delta_{\rm cw,c} = \frac{F_{\rm cw,c} h_{\rm cw}}{E b_{\rm eff,cw} t_{\rm cw}} \tag{4}$$

$$R_{\rm c,cw} = \frac{Eb_{\rm eff,cw} t_{\rm cw}}{h_{\rm cw}}$$
(5)

where  $\Delta_{cw,c}$  is displacement of compressive column web;  $F_{cw,c}$  is pressure on column web;  $h_{cw}$  is height of column web;  $b_{eff,cw}$  is effective compressive width of column web,  $b_{eff,cw} = 2t_a + r_a + 5(t_{cf}+s)$ ;  $t_{cw}$  is thickness of column web;  $r_a$  is arc radius at corner of angle toe, when column section is rolled,  $s = r_a$ , when column section is welded,  $s = h_{fc}$ ,  $h_{fc}$  is weld leg size at junction between column flange and column web.

#### 2.3. The Initial Stiffness of Tensile Column Web

The derivation process of initial stiffness of compressive column web were similar with tensile column web, except for difference of the effective width. The tensile effective width of column web was shown in previous work by [4], and the displacement and the initial stiffness of tensile column web were shown in Eq. (6) and Eq. (7) below:

$$\Delta_{\rm cw,t} = \frac{F_{\rm cw,t} h_{\rm cw}}{E b'_{\rm eff,cw} t_{\rm cw}} \tag{6}$$

$$R_{\rm t,cw} = \frac{E \dot{b}_{\rm eff,cw} t_{\rm cw}}{h_{\rm cw}}$$
(7)

where  $\Delta_{cw,t}$  is displacement of tensile column web;  $F_{cw,t}$  is pressure on column web;  $b'_{eff,cw}$  is effective tensile width of

# 2.4. The Initial Stiffness of Bending Column Flange

The pressure on tensile beam flange was transfer to column flange by bolt, and rotation disappeared at junctions between column web and column flange. So that bending column flange was regarded as T shape member, be shown in Fig. (1) below:



Fig. (1). The T shape member.



Fig. (2). Calculated model of bending column flange.

Considering pretension force of high strength bolt is very huge, first row bolts closed to column web were regarded as fixed end approximatively. The T shape member was regarded as beam fixed at both ends to calculate, the calculated model of T shape member was shown in Fig. (2) above [5].

The displacement at junction between column web and column flange was equivalent to displacement at midspan of beam under concentrated force at midspan of beam plus elongation of pretension bolt. The displacement is got by Eq. (8) and Eq. (9), and the initial stiffness is shown in Eq. (10).

$$\Delta_{\rm cf,t} = \frac{F_{\rm cw,t} l_{\rm cf}^3}{192EI_{\rm cf}} + \Delta_{\rm b,t}^{\prime}$$
(8)

$$\Delta_{b,t} = \frac{F_{cw,t} t_{cf}}{11 E A_b}$$
(9)

$$R_{t,cf} = \frac{1}{\frac{l_{cf}^{3}}{192EI_{cf}} + \frac{t_{cf}}{11EA_{b}}}$$
(10)

where  $\Delta_{cf,t}$  is displacement at junction between column web and column flange under tension on column flange;  $F_{cw,t}$  is tension on column web;  $l_{cf}$  is calculated length of tensile column flange,  $l_{cf} = 2a$ , *a* is distance of bolt closest to column web;  $I_{cf}$  is moment of inertia of column flange section,  $I_{cf} = b_{eff,t} t_{cf}^3/12$ ,  $b_{eff,t}$  is effective width of bending column flange, be shown in previous work by [4],  $b_{\text{eff},t} = 2\pi a$ ;  $\Delta_{b,t}$  is elongation of pretension bolt.

# 2.5. The Initial Stiffness of Tensile Angle

The calculated model of angle is shown in Fig. (3). Considering pretension of bolt is very huge, the situation at bolt is regarded as fixed end [6]. The angle displacement is shown in Fig. (4).



Fig. (3). Calculated model of angle.



Fig. (4). Calculated drawing of angle.

Calculated drawing of angle is shown in Fig. (4), the member end force caused by member end displacement is gotten by principle of virtual work,  $\{x_1, x_2, x_3\}^T$ . Due to similar between length of angle toe and thickness of angle toe, shearing deformation should be considered [7]. The derivation process are shown in Eq. (11)~Eq. (14) below:

$$\{x_{1}, x_{2}, x_{3}\}^{\mathrm{T}} = \left\{\alpha, \alpha\beta, \frac{1}{3}g_{1}\alpha\right\}^{\mathrm{T}}$$
(11)

$$\alpha = \frac{3EI_a h_1 \theta}{\beta g_2^3 - g_1 g_2^2} \tag{12}$$

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$$\beta = \frac{1}{g_2^2} \left( \frac{4}{3} g_1 g_2 + \frac{1}{3} g_1^2 \right)$$
(13)

$$R_{\rm t,a} = M/\theta = \frac{3EI_{\rm a}\beta g_1 h_1}{\beta g_1 g_2^3 - g_1^2 g_2^2}$$
(14)

where  $I_a$  is moment of inertia of angle;  $g_2$  is calculated length of junction between top angle and beam flange,  $g_1 = g_b - D/2 - t_a/2$ ,  $g_b$  is distance from junction between top angle and column flange to first row bolt hole center of beam, *D* is diameter of bolted cap;  $g_2 = g_c - D/2 - t_a/2$ ,  $g_c$ is distance from junction between top angle and column flange to first row bolt hole center of column;  $h_1$  is distance between top angle and seat angle,  $h_1 = h_b + t_a$ ,  $h_b$  is height of beam.

#### 2.6. The Initial Stiffness of Semirigid Connection

Due to above-mentioned four tensile region are linked with series pattern, the initial stiffness of tensile region is shown in Eq. (15) below:

$$\frac{1}{R_{\rm t}} = \frac{1}{R_{\rm t,b}} + \frac{1}{R_{\rm t,cf}} + \frac{1}{R_{\rm t,cw}} + \frac{1}{R_{\rm t,a}}$$
(15)

where  $R_t$  is initial stiffness of tensile region.

The angle rotation caused by connection moving included rotation of compressive region and rotation of tensile region. The derivation process are shown in Eq. (16)~Eq. (18) below:

$$R_{\rm ki} = \frac{M}{\theta} \tag{16}$$

$$\theta = \frac{\Delta_{\rm t} + \Delta_{\rm c,cw}}{h_{\rm 1}} \tag{17}$$

$$M = F_{\rm bf,t} \times h_1 \tag{18}$$

where  $\theta$  is total angular rotation;  $\Delta_t$  is linear displacement in tensile region,  $\Delta_t = F_{bf,t}/R_t$ ,  $F_{bf,t}$  is tension on beam flange;  $\Delta_{c,cw}$  is linear displacement in compressive region,  $\Delta_{c,cw} = F_{bf,c}/R_{c,cw}$ ,  $F_{bf,c}$  is pressure on beam flange; *M* is moment of beam.

The Eq. (16) and Eq. (17) are substituted into The Eq. (18), and the result is shown in Eq. (19) below.

$$R_{\rm ki} = \frac{h_1^2}{\frac{1}{R_{\rm t}} + \frac{1}{R_{\rm c,cw}}}$$
(19)

# **3 THE EXAMPLE ANALYSIS AND FORMULA FIT-TING**

The dimensions of connections was shown in Table 1 below.

The results of initial stiffness by Eq. (19) were shown in Table 2. The major effective factor of initial stiffness is angle tensile initial stiffness ( $R_{t,a}$ ) from Table 2, and the fitted formula is shown in Eq. (20) considering other effective factor of initial stiffness.

$$R_{\rm ki} = \frac{0.19Eb_{\rm b}t_{\rm a}^3h_{\rm b}^2t_{\rm cw}(8t_{\rm a} + 5t_{\rm cf})}{1.1b_{\rm b}t_{\rm a}^3h_{\rm cw} + 4.1d_{\rm e}^3(8t_{\rm a} + 5t_{\rm cf})t_{\rm cw}}$$
(20)

where *E* is elastic modulus of steel;  $b_b, h_b$  are width of beam and height of beam respectively;  $t_a$  is thickness of angle;  $h_{cw}, t_{cf}, t_{cw}$  are height of column web, thickness of column flange and height of column web respectively.  $d_e$  is effective diameter of bolt.

No.	Column Section (mm)	Beam Section (mm)	Angle Section (mm)	BoltDiameter (mm)	
1	HW200X200X8X12	HW150X150X7X10	2L100X80X8	16	
2	HW250X250X9X14	HW200X200X8X12	2L100X80X10	16	
3	HW300X300X10X15	HW250X250X9X14	2L140X90X12	16	
4	HW350X350X12X19	HW300X300X10X15	2L140X90X14	22	

 Table 2.
 Initial stiffness of connection (Calculaed by Eq. (19)).

No	$R_{t,b}(N \cdot m/rad)$	$R_{c,cw}(kN \cdot m/rad)$	$R_{t,cw}(kN \cdot m/rad)$	$R_{t,cf} kN \cdot m/rad)$	R <sub>t,a</sub> (kN·m/rad	$R_t$ (kN·m/rad)	<i>R</i> <sub>ki</sub> kN·m/rad)
1	4140	1273	891	1644	115	94	2082
2	3450	1253	877	1184	427	217	5897
3	6133	1305	913	1099	247	161	11369
4	4743	1545	1082	1569	564	282	18747

 Table 3.
 Initial stiffness of connection (fitted value).

No.	<i>E</i> (N/mm <sup>2</sup> )	<b>b</b> <sub>b</sub> ( <b>mm</b> )	<b>h</b> ь ( <b>mm</b> )	t <sub>a</sub> (mm)	t <sub>cf</sub> (mm)	t <sub>cw</sub> (mm)	h <sub>cw</sub> (mm)	d <sub>e</sub> (mm)	R <sub>ki</sub> (kN·m/rad)	Fitted value/ Eq.(19)
1	206000	150	150	8	12	8	176	16	2165	1.04
2	206000	200	200	10	14	9	222	16	6015	1.02
3	206000	250	250	12	15	10	270	16	11711	1.03
4	206000	300	300	14	19	12	312	22	18747	1.00

#### Table 1. Dimension of connection models.

The initial stiffness in Table 1 are calculated with Eq. (20) again and the results are analyzed comparatively with initial stiffness in Table 2. The Table 3 show us every parameter value, results and ratio of fitted value and Eq. (20). The Eq. (20) is more convenient than Eq. (19) from Table 3, and the error is in the allowed range in practical design [8].

#### CONCLUSION

In this paper, according to stress characteristic and failure mode of top and seat angle semirigid connections, the component method are used to derivate initial stiffness and get analysis formula and fitted formula. Those results were analyzed with test data and verify correction of formulas. The results provide good theoretical foundation for practical design method of project.

# **CONFLICT OF INTEREST**

The authors confirm that this article content has no conflict of interest.

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