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RESEARCH ARTICLE

Influence of Local Site Effects on the Typological Fragility Curves for Class-Oriented Masonry Buildings in Aggregate Condition

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Abstract:

Background:

The Municipality of Baranello, located in the province of Campobasso, is considered as a reference case study for implementing a systematic procedure for the assessment of local site effects. The territory is characterised by a heterogeneous soil, made of a basal interval formed in large part by outcrops found along with the middle-basal bands of the local slopes, which is particularly sensitive to geo-hazard effects.

Aims:

The present work aims to investigate the influence of local site effects on the seismic vulnerability of an oriented- class of masonry building compounds. The historical buildings surveyed are mainly made up of rough stone masonry, particularly vulnerable to seismic phenomena. The buildings are appropriately grouped in typological classes based on the CARTIS form and then subdivided into different categories according to the EMS-98 scale based on their geometrical and structural characteristics.

Methods:

The performed vulnerability analysis is based on a vulnerability-index method conceived for buildings grouped in aggregate condition. Moreover, the expected damage scenario is properly evaluated based on a specific attenuation law to take into consideration the effect produced by a reference scenario event. Furthermore, the local site conditions have been evaluated based on a 1D half space approach in order to predict the possible increment of the expected damage due to the soil stratigraphy.

Results:

The local site conditions provide a global increment of the expected vulnerability of 33% with respect to the case in which the geo-hazard effects are neglected. In this activity framework, typological fragility curves are derived for comparing the exceeding probability thresholds for the examined building classes.

Conclusion:

The proposed research represents an important starting point for large-scale vulnerability assessment since it provides a simplified and comprehensive method for predicting seismic damage scenarios of historic center considering local site effects.

Keywords: Vulnerability assessment, Masonry building compounds, Large-scale analysis, Damage scenarios, Site effects, Attenuation law, Vulnerability curves, Fragility curves.

Article History

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1. INTRODUCTION

The tragic earthquakes that occurred in Italy in recent years were a consistent testimony of low seismic performance levels of historical centres of many municipalities due to a series of

deficient factors, such as the age of buildings, the poor quality of materials and the scarce maintenance state of constructions, which led towards significant seismic damages.

Generally, in an overall overview, the Vulnerability (V), combined through a multi-factorial procedure with other two parameters, such as Exposure (E) and Hazard (H), leads to the definition of the seismic risk, which can have a direct or indirect influence on a specific site. Reducing the expected

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level of seismic risk means assessing the seismic capacity of buildings sample towards earthquakes (Vulnerability), the amount of human and material resources that could be lost in case of seismic events (Exposure) and the intensity and recurrence of earthquakes (Hazard).

The seismic vulnerability of a given built area denotes the expected amount of damage caused by an earthquake with a specific Intensity Measurement (IM) [1]. In this perspective, the seismic vulnerability assessment of an urban centre is devoted to estimate the propensity of the built-up area to undergo seismic phenomenon without failures, which are responsible for huge economic losses and casualties.

In this framework, focusing on the urban scale, rapid urbanization has dramatically increased the vulnerabilities of the exposed material assets of urban dwellers in densely populated areas. The high population growth and the presence of a conspicuous number of buildings designed without anti-seismic regulations significantly have expanded the problem of seismic safety condition in urbanized areas [2, 3].

At urban-scale, the identification of the most vulnerable class-oriented buildings is not a simple task due to their structural heterogeneity and complexity. In general, the susceptibility of buildings to be damaged by a scenario event depends on many aspects, such as (i) The construction type, (ii) The quality of materials used, (iii) The construction methods and (iv) The preservation state. Many studies [4 - 6] have highlighted how the lack of these peculiarities makes structures ineffective against seismic actions. In particular, it is worth noting how especially masonry buildings located in historic centres are often characterized by a static inadequacy, mainly due to unsuitable construction techniques, which do not guarantee an adequate safety level. To this purpose, an inventory of building typologies is an essential procedure for the acquisition of preliminaries data aiming at performing large-scale seismic vulnerability [7, 8].

The available strategies usually take into consideration survey forms [9, 10] to collect several seismic parameters of historical buildings, i.e. the seismic-resistant system type to lateral loads, the structural regularity, the maintenance conditions and the presence of existing damages. The application of these survey forms allows to fully understand various structural typologies located within heterogeneous urban centres [4]. Thus, in this perspective, the impact of an earthquake can be assessed in terms of expected losses facilitating risk management purposes. In fact, the formulation of an adequate earthquake loss model in a given region is not only essential for economic purpose due to the impact of future earthquakes, but it is also important for risk mitigation measures. A specific seismic loss model allows to predict the expected damage of the built environment due to a specific event and could be particularly important for a systematic action aimed at emergencies planning in order to safeguard people and historical heritage of a given site [11 - 13].

Nowadays, in the framework of seismic risk assessment, another important aspect regards the evaluation of the geological effects in order to have a better and correct forecast of the expected structural damage [14].

In this circumstance, the soil layers can amplify or reduce the effect of the seismic waves at ground surface. Site effects are, thus, dangerous when the amplification of seismic waves in surface geological layers occurs since it produces an increase of expected physical damage of some building classes even in case of low-intensity/magnitude earthquakes. In fact, surface motion can be strongly amplified if geological conditions are unfavourable [15].

Generally, all these phenomena are called site or local amplification effects. The characterization of site effects can be carried out considering the ground model as a 1D or 2D half-space. In a 1D analysis only the depth of the half-space is taken into consideration, neglecting the lateral confinement effect, while the more accurate 2D analysis considers the significant volume of soil in the longitudinal and vertical directions. The study of site effects can be conducted using the above two distinct procedures in either the time domain or the frequency domain. In the first case, the local amplification factor is estimated by means of the time sequence (time history) at the bedrock with respect to the amplification at the ground surface. In the second case, the amplification coefficient is determined considering the Fourier amplitude spectra based on a proper response spectrum [16].

A reliable and easy method for large scale analysis developed in [14, 16] allows to estimate the macro seismic intensity increment derived from a specific soil category, so as to properly define, taking into account the local amplification factors, the global vulnerability of building stocks according to the EMS-98 scale. As proposed in [14], the site conditions have been evaluated through the definition of the acceleration design response spectrum of a given geographical area, accounting for the soil type based on the shear waves, V_{s30} , and neglecting the real soil stratigraphy [17].

Thus, starting from the study [14], the present research aims to propose a systematic and simplified procedure for assessing geological site effects in an organic and comprehensive way. The proposed methodology provides an integrated assessment of seismic vulnerability considering the influence of site effects on the structural response of a sample of buildings. The main aspect of this methodology is its straightforward execution, since it is applicable to any seismic area considering as input parameters the site stratigraphy and the reference seismic event.

Based on these considerations, the municipality of Baranello, in the province of Campobasso, has been selected as a reference case study to evaluate the possible damages of the built-up area under seismic events considering local site effects through a 1D half-space ground model in the time domain. The main goal of this work is to investigate the influence of soil condition on the seismic behaviour of typical masonry aggregates with the final target to plot the damage scenarios in terms of typological fragility curves derived from an empirical-based approach that correlates the macro seismic intensity of the site to the expected seismic acceleration.

2. MATERIALS AND METHODS

2.1. Survey and Analysis Methods

2.1.1. Historical Background of the Municipality of Baranello

Baranello (Fig. 1) is a small town located in the province of Campobasso in the Molise Region of Italy. The municipality has 2759 inhabitants and rises at 610 m above sea level with an extended area of 25 km². The town, which has medieval origins, is bordered by the towns of Busso, Colle d'Anchise, Spinete and Vinchiaturo.

There there are limited historical informations about the evolution of the centre over the time. The village was mentioned for the first time in the 14th century as a possession of Capece Galeota dating back to Norman ages. Only in 1591 the feud was sold by the Carafa family to the Marquis and then to Angelo Barone. Until the 19th Century, Baranello was part of the Aragonese domain and, subsequently, of the Ruffo family. Within the village, an example of Norman architecture was found in the Ruffo castle, owned by the homonymous family until the 19th century. The castle was built at the highest point of the ancient village, performing its function of defence and control of the entire territory.

Nowadays, the territory presents the characteristics of a mountain centre: there is the predominance of narrow and steep streets that become wider and easier towards the area of a new settlement. The urban centre is mainly characterized by houses

which maintain their original appearance and are located around the church and along the main streets, while the modern buildings are placed in other districts belonging to the municipal territory [18].

2.1.2. Typological and Structural Characterization of the Urban Centre

The structural and typological characterization of urban centres is an important step in order to have specific census data of the main building typologies exposed at the seismic risk.

In the examined specific context, the CARTIS form has been used in order to detect the prevalent ordinary building typologies in the historical centre of Baranello. The CARTIS form has been conceived by the PLINIVS research centre of the University of Naples "Federico II" in collaboration with the Italian Civil Protection Department (DPC) during the ReLUIS 2014-2016 project "Development of a systematic methodology for the assessment of exposure on a territorial scale based on the typological/structural characteristics of buildings" [12, 19].

The form is mainly divided into four sections: Section 0, for the identification of the municipality and the sectors identified therein; Section 1, for the recognition of each of the relevant typologies characterizing the generic sub-sector of the assigned municipality; Section 2, for the detection of general characteristics of each typology of constructions; Section 3, for the characterization of structural elements of all individuated construction typologies. Focusing on the case study, the historical centre of Baranello is composed of a unique compartment, named C01 and made of 300 buildings (Fig. 2).

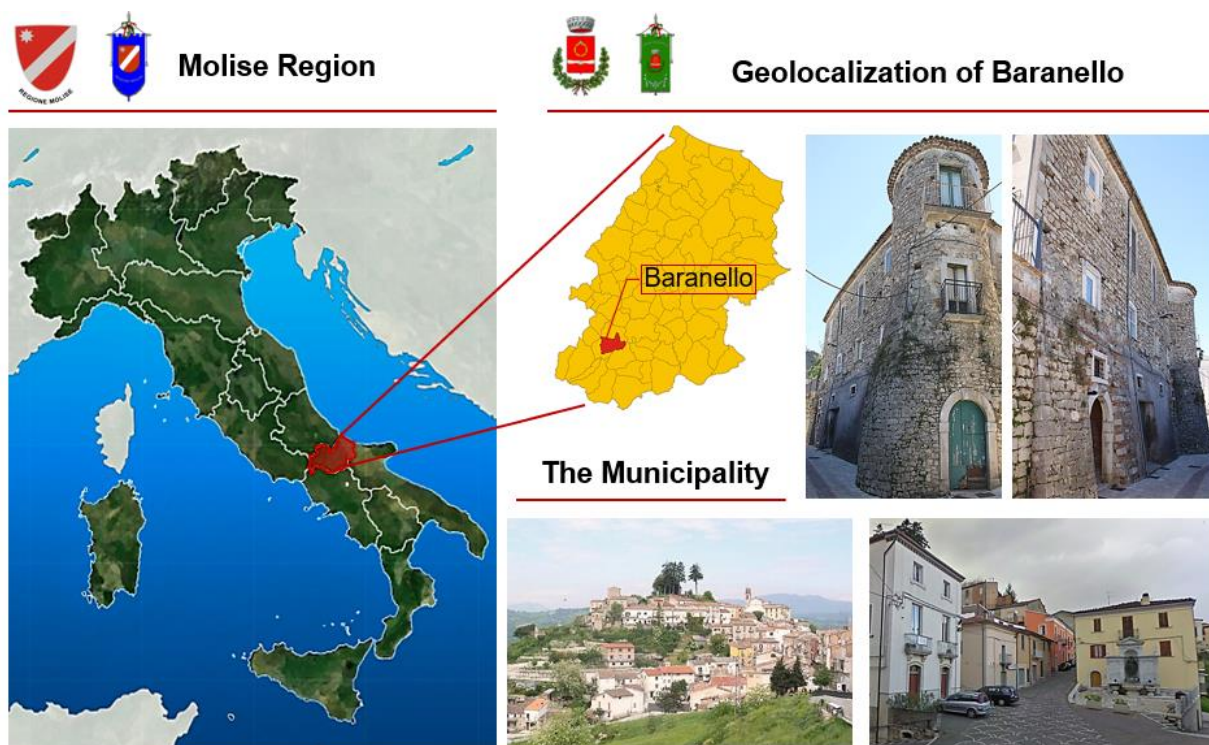


Fig. (1). Geo-localization of the municipality of Baranello.



Fig. (2). C01 compartment of the historical centre of Baranello.

In the examined sector, most of the buildings have been built using rough-hewn stones assembled according to the technique of sack or mixed walls, which in some cases have greatly affected the characteristics and quality of constructions, offering substantial deficiencies in terms of global response towards seismic actions. In particular, the houses characterising the historical centre are composed of masonry walls with an average thickness of 0.65 m and an average inter-storey height of 3.50 m. Apart masonry vaults, the horizontal structures, as well as roofs, are generally made of either timber or steel beams (Fig. 3).

The data collected through the CARTIS form has been elaborated statistically to provide indications on constructive age, number of storeys, average surface area and wall type of the sample of buildings surveyed within the municipality examined. The results obtained are plotted in the cumulative distributions reported in Fig. (4).

From the data collected on the 300 inspected buildings, the prevailing typological class is the MUR2 (rough-hewn stone)

one, which has been detected in 75% of the cases (225 buildings). Moreover, about 10% of the sample are abandoned (acronym A. B. in Fig. (4)).

2.1.3. Derivation of the Damage Probability Matrices (DPM)

After the earthquake occurred in Friuli in 1976, the investigation of post-earthquake damage to ordinary buildings has become a crucial priority for emergency management of the Italian historical cities. To this purpose, the Post-Earthquake Damage and Safety Assessment (AeDES) form was introduced in 1997 as an efficient operational tool, recognized by the Italian Civil Protection Department for the detection and management of post-earthquake emergencies (Fig. 5) [20].



Fig. (3). Street views of the characteristic building typologies within the historical centre of Baranello.

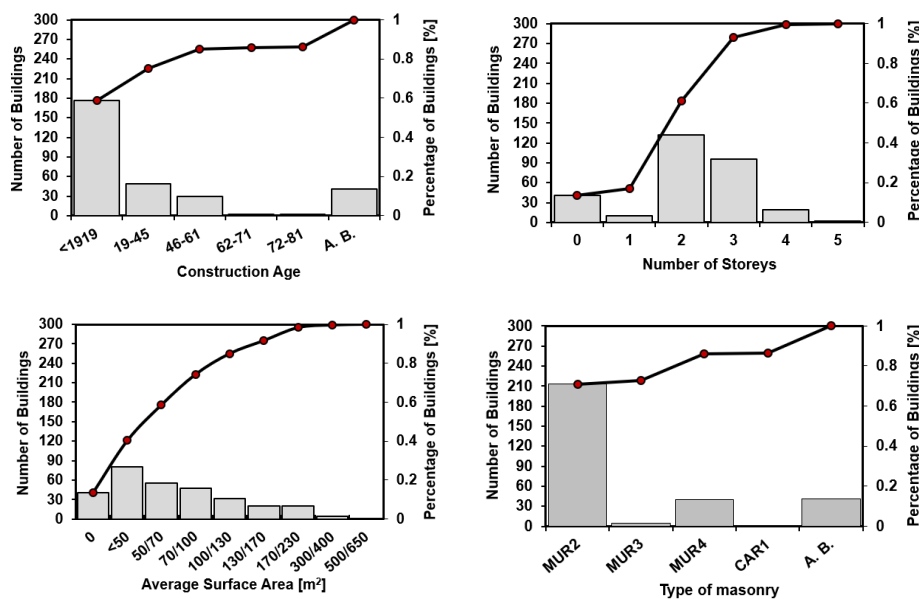


Fig. (4). Preliminary construction characteristics of the buildings surveyed in the municipality of Baranello.

		Masonry buildings								Other structures			Masonry structures				Masonry structures				
		Unknown				Regular layout and good quality (Blocks, bricks, squared stone...)				R.c. frames			Type I		Type II		Isolated columns				
Vertical structures		Irregular layout or bad quality (rubble stones, pebbles...)		Regular layout and good quality (Blocks, bricks, squared stone...)		R.c. shear walls			Steel frames		Without tie rods or tie beams		With tie rods or tie beams		Without tie rods or tie beams		With tie rods or tie beams		Isolated columns		
		W/O tie rods or tie beams	With tie rods or tie beams	W/O tie rods or tie beams	With tie rods or tie beams	Isolated columns	Mixed	Strengthened	Irregular	Regular	Without tie rods or tie beams	With tie rods or tie beams	Without tie rods or tie beams	With tie rods or tie beams	Without tie rods or tie beams	With tie rods or tie beams	Isolated columns	Mixed	Strengthened		
Horizontal Structures		A	B	C	D	E	F	G	H	REGULARITY			Roof		Isolated columns						
1 Not identified		<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	1 Plan and elevation	<input type="checkbox"/>	<input type="checkbox"/>	1 <input type="checkbox"/> Thrusting heavy	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>			
2 Vaults without tie rods		<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	2 Infill distribution	<input type="checkbox"/>	<input type="checkbox"/>	2 <input type="checkbox"/> Non thrusting heavy	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>			
3 Vaults with tie rods		<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>				3 <input type="checkbox"/> Thrusting light	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>			
4 Beams with flexible slab (wooden beams with a single layer of wooden planks, beams and shallow arch vaults...)		<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>				4 <input type="checkbox"/> Non thrusting light	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>			
5 Beams with semirigid slab (wooden beams with a double layer of wooden planks, beams and hollow flat blocks...)		<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>				<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>			
6 Beams with rigid slab (r.c. floors, beams well connected to r.c. slabs...)		<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>				<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>			

Damage level - extension	DAMAGE (1)												EXISTING SHORT TERM COUNTERMEASURES					
	D4-D5 Very Heavy			D2-D3 Medium-Severe			D1 Light			Null			None	Removal	Ties	Repair	Popping	Barriers or passage protection
	> 2/3	1/3 - 2/3	< 1/3	> 2/3	1/3 - 2/3	< 1/3	> 2/3	1/3 - 2/3	< 1/3	Null	A	B	C	D	E	F		
1 Vertical structures	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	
2 Floors	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	
3 Stairs	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	
4 Roof	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	
5 Infills and partitions	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	
6 Pre-existing damage	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	

Damage level	Countermeasures
A USABLE building	<input type="checkbox"/>
B UNUSABLE building (totally or partially), but USABLE after short term countermeasures	<input type="checkbox"/>
C PARTIALLY UNUSABLE building (1)	<input type="checkbox"/>
D TEMPORARILY UNUSABLE building requiring a more detailed investigation	<input type="checkbox"/>
E UNUSABLE building	<input type="checkbox"/>
F UNUSABLE building due to external risk (1)	<input type="checkbox"/>

Fig. (5). Main section of the AeDES form.

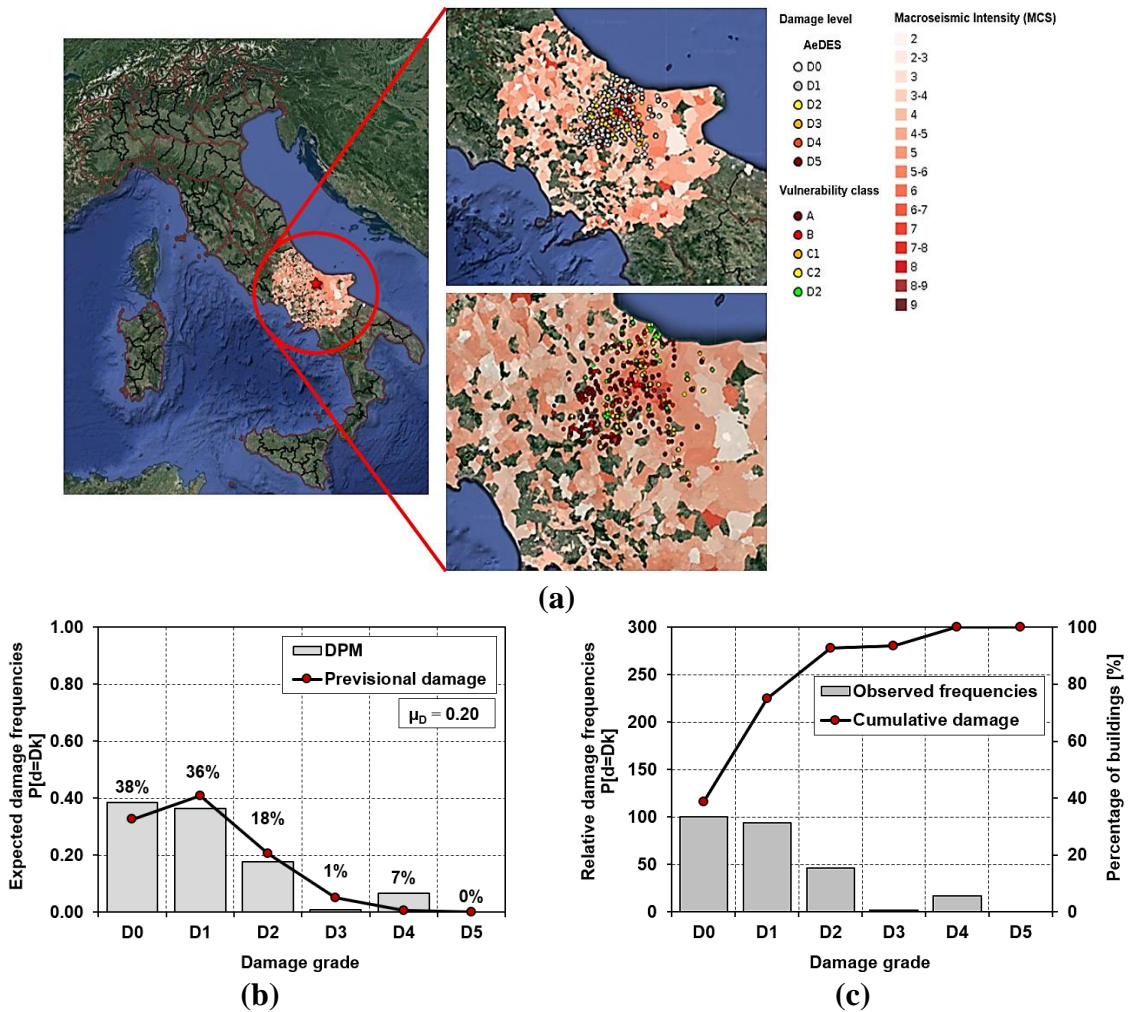


Fig. (6). Damage assessment: (a) Da.D.O database, (b) damage probability matrices and (c) cumulative damage distribution.

Subsequently, in 2014, the Italian Civil Protection Department promoted a new scientific project with the aim of creating a new database at national level to simulate seismic risk scenarios. The project, in collaboration with the Eucentre Foundation (European Centre for training and research of Earthquake Engineering), led to the development of the Web-GIS platform called Da.D.O (Observed Damage Database) [21]. This tool represents an important support for the scientific community since it collects and catalogues all the data, surveyed through the AeDES form, about both structural damages and main characteristics of inspected buildings detected after last past earthquakes (Friuli 1976, Irpinia 1980, Abruzzo 1984, Umbria and Marche 1997, Pollino 1998, Molise and Puglia 2002, Emilia-Romagna 2003, L'Aquila 2009 and Emilia-Romagna 2012). Thus, for the quantification of the observed damage, using the Da.D.O database and exploiting the information collected in the linked AeDES form, the Damage Probability Matrices (DPM) have been statistically processed using the binomial distribution function according to the following equation [12, 22]:

$$p_k = \frac{5!}{k!(5-k)!} \times \left(\frac{\mu_D}{5}\right)^k \times \left(1 - \frac{\mu_D}{5}\right)^{5-k} \quad (1)$$

Where k denotes the damage threshold variable from 0 to 5 according to the EMS-98 scale and μ_D represents the weighted average of damages. In this context, the event occurred in Molise in 2002 (with epicentre in Bonefro) has been considered in order to collect the main information regarding typological classes, damage level occurred and macroseismic

intensity of the study area. In Fig. (6) a representation of the DPM has been done.

Afterwards, based on the examined typological classification of the surveyed buildings, a typological correlation has been conducted among CARTIS, Da.D.O and EMS-98 in order to differentiate the prevalent typological vulnerability classes [12] based on the vertical bearing structure. The attribution of a vulnerability class takes place by means of acronyms identifying the materials characterizing the vertical structure. In this case, the transition from one vulnerability class to another, which provides the reduction of the vulnerability level derived from the CARTIS form (from irregular masonry stone buildings, MUR1, to reinforced concrete framed structures, RC1), has been based on the macroseismic method EMS-98, which incorporates all the typological classes derived from the AeDES form into four typological classes, from A (the worst) to D (the best). Thus, the typological vulnerability classes of the investigated urban area are shown in Fig. (7).

3. RESULTS

3.1. Vulnerability-Based Approach

A seismic vulnerability assessment at urban scale has been implemented in order to evaluate the propensity at the damage of buildings exposed at earthquakes. In this perspective, a vulnerability index-based method has been adopted. The peculiarity of this method, proposed in [23], is the possibility of investigating the seismic vulnerability of building compounds through the easy vulnerability form depicted in Table 1.

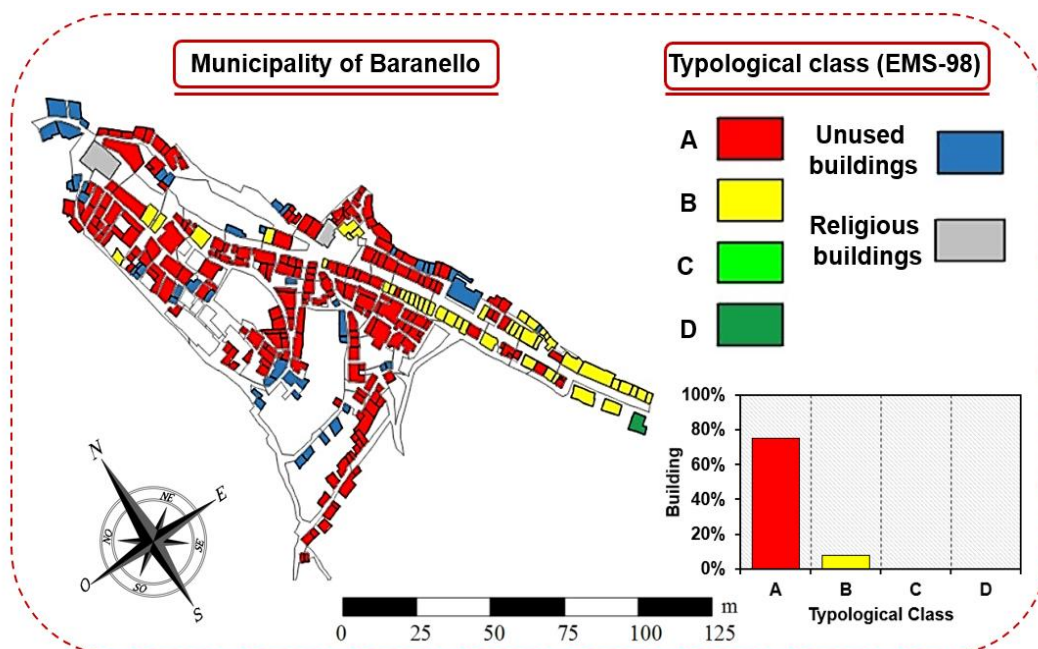


Fig. (7). Typological vulnerability classes of the municipality of Baranello derived from the EMS-98 scale.

Table 1. Vulnerability form conceived for historical masonry building compounds.

Parameters	Class Score, S_i				Weight, W_i
	A	B	C	D	
1. Organization of vertical structures	0	5	20	45	1.00
2. Nature of vertical structures	0	5	25	45	0.25
3. Location of the building and type of foundation	0	5	25	45	0.75
4. Distribution of plan resisting elements	0	5	25	45	1.50
5. In-plane regularity	0	5	25	45	0.50
6. Vertical regularity	0	5	25	45	1.00
7. Type of floor	0	5	15	45	1.00
8. Roofing	0	15	25	45	0.75
9. Details	0	0	25	45	0.25
10. Physical conditions	0	5	25	45	1.00
11. Presence of adjacent building with different height	-20	0	15	45	1.00
12. Position of the building in the aggregate	-45	-25	-15	0	1.50
13. Number of staggered floors	0	15	25	45	0.50
14. Structural or typological heterogeneity among adjacent S.U.	-15	-10	0	45	1.20
15. Percentage difference of opening areas among adjacent facades	-20	0	25	45	1.00

This form, based on the original method proposed by Benedetti and Petrini some decades ago [24], has been appropriately extended to masonry building aggregates in 2015 [23] using five new additional parameters, which take into account the effects of mutual interaction among Structural Units (S.U.s) under earthquakes.

Methodologically, the vulnerability index, I_v , is evaluated for each S.U. as the weighted sum of the class selected for each of the 15 parameters listed in Table 1 multiplied by the respective weight. The estimated parameters are grouped in 4 vulnerability classes (from A, the best, to D, the worst), characterised by a specific score (also with a negative sign in case of vulnerability reduction). To each parameter is assigned a given weight, W_i , which is variable from a minimum of 0.25, for the less important parameters, up to a maximum of 1.50, for the most important factors of the form [2, 23].

Thus, the vulnerability index, I_v , is calculated according to the following equation:

$$I_v = \sum_{i=1}^{15} S_i \times W_i \tag{2}$$

where, S_i , is the score associated to the i -th parameter, and W_i is the i -th parameter weight.

Subsequently, the vulnerability index value I_v is normalized in the range (0-1), taking the notation of V_i , by means of the following equation:

$$V_i = \left[\frac{I_v - (\sum_{i=1}^{15} S_{\min} \times W_i)}{\sum_{i=1}^{15} [(S_{\max} \times W_i) - (S_{\min} \times W_i)]} \right] \tag{3}$$

where: I_v is the vulnerability index deriving from Eq. (2);

($S_{\min} \times W_i$), equal to -125.50, represents the sum of scores associated to the vulnerability class A of each parameter multiplied by respective weights; ($S_{\max} \times W_i$), equal to 495.00, represents the sum of scores associated to the vulnerability class D of each parameter multiplied by the respective weights.

Thus, the distribution of the vulnerability indices of typological classes examined has been represented in Fig. (8).

The results have shown that the distribution of the vulnerability is homogeneous, with an expected medium-high vulnerability level enclosed in the range (0.4-0.6) for typological class A, ($V_{i, \text{mean}}=0.55$). Instead, for the analysed typological class B, the expected frequency is in the range (0.2-0.4), which corresponds to a moderate vulnerability level ($V_{i, \text{mean}}=0.36$).

Subsequently, the typological vulnerability curves [2, 11, 1] have been evaluated in order to estimate the propensity of damage of the building stock varying the level of macroseismic intensity, as reported in Fig. (9).

Mathematically, these curves have been derived according to the following relationship:

$$\mu_D = 2.5 \times \left[1 + \tanh \left(\frac{I_{\text{EMS-98}} + 6.25 \times V_i - 13.1}{Q} \right) \right] \tag{4}$$

where V_i is the normalized vulnerability index, $I_{\text{EMS-98}}$ is the seismic hazard expressed in terms of macroseismic intensity and Q is the ductility factor, which describes the ductility of a certain typological class and assumes, in this specific study, the value of 2.3, as suggested in [14].

In Fig. (9) the vulnerability curves have been associated with other four curves, which represent the upper and lower bounds ($V_m - \sigma$; $V_m + \sigma$; $V_m + 2\sigma$; $V_m - 2\sigma$) of the statistical interval of the expected damage [2, 12].

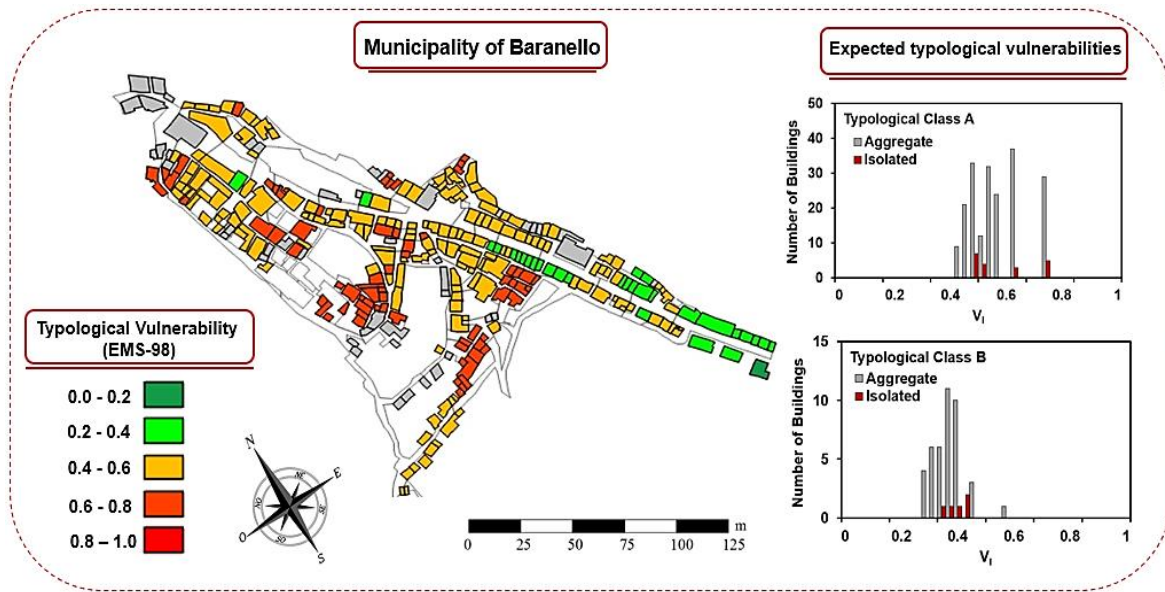


Fig. (8). Vulnerability Distribution of Building Typological Classes in the Examined Municipality.

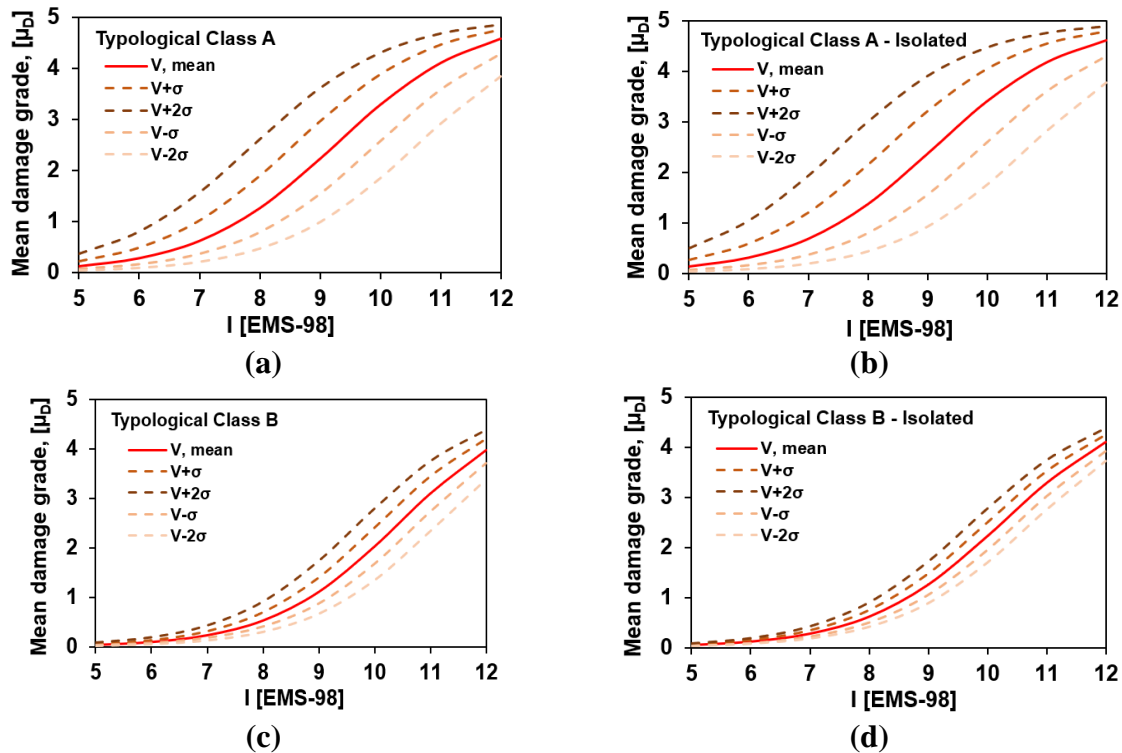


Fig. (9). Typical vulnerability curves of building classes in aggregated (a, c) and isolated (b, d) conditions detected in Baranello.

3.2. Damage Scenario

In the present study, the severity of the seismic damage scenario effects has been analysed by predictive analyses using a proper seismic attenuation law. Generally, the prediction of the seismicity of a specific site can be evaluated by adopting appropriate seismic attenuation laws, which are empirical formulations calibrated on the statistical data (instrumental or

macroseismic) analysis of earthquakes occurred. The prerogative of these formulations is to estimate the value of a main synthetic seismic parameter (*i.e.*, accelerations, velocity, seismic intensity, *etc.*) based on intrinsic factors deriving from the earthquake itself, such as magnitude (M_w) and epicentre (R) or hypocentre (h) distances. Several studies [25, 26] have been conducted to develop specific attenuation laws for a given area,

generated in terms of Spectral accelerations (S_a) and Peak Ground Accelerations (PGA) or in terms of seismic intensity (MMI), considering a statistical regression of the instrumental data recorded by the earthquakes occurred.

Herein, the propagation of the seismic motion has been described according to the attenuation law proposed by Esteva *et al.* [27]:

$$I_{EMS-98} = 1.45 \times M_w - 2.46 \times \ln(R + 8.16) \quad (5)$$

Where M_w is the moment magnitude and R is the site-to-source distance (measured in Km).

Therefore, the analysis conducted is based on an empirical-forecast method, where the probable damage scenarios are estimated by the disaggregation of the seismic risk obtained combining n-sources. In detail, a deterministic approach has

been used by selecting as reference earthquakes, according to the Parametric Catalogue of Italian Earthquakes (CPTI15) [28], 3 past events of increasing magnitude (4, 5 and 6), that have influenced the case study municipality. After these moment magnitudes have been selected, the definition of different epicentre distances (in the range from 5 to 35 Km) has allowed to plot the expected damage scenarios (Fig. 10).

In particular, based on the attenuation law above mentioned, the macroseismic intensities have been correlated to the earthquake magnitudes on the basis of Eq. (5), leading towards the 9 damage scenarios reported in Table 2.

For the sake of representation, the damage scenarios derived from Esteva *et al.*'s attenuation model considering $R=5$ Km, which is the most unfavourable case, have been presented in Fig. (11).

Table 2. Correlation between moment magnitude, M_w , and macroseismic intensity, I_{EMS-98} .

Magnitude M_w	Macroseismic Intensity I_{EMS-98} – Esteva <i>et al.</i> (1964) [42]		
	R = 5 Km	R = 17 Km	R = 35 Km
4	X	VII	V
5	XI	VIII	VII
6	XII	X	VIII

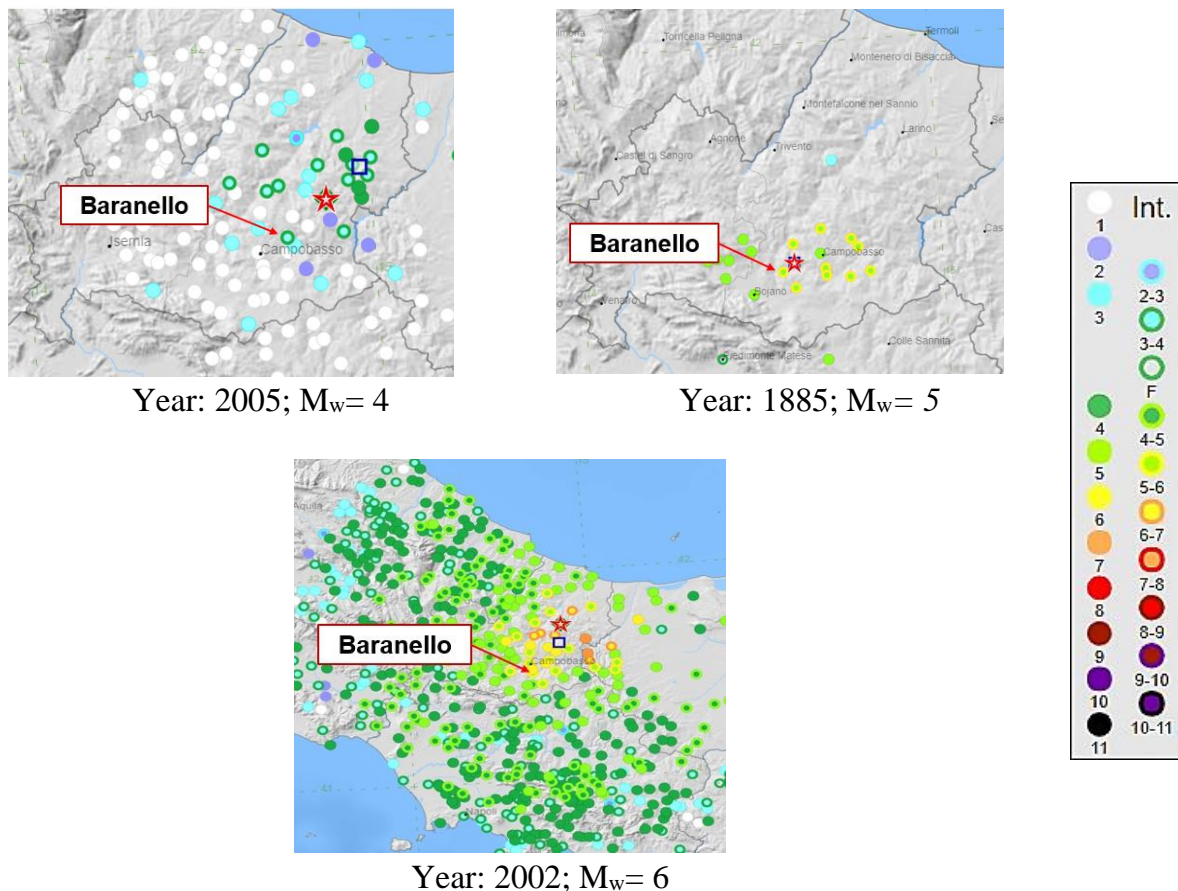


Fig. (10). The selected seismic events for the case study area.

For an exhaustive and immediate representation of the damage, the DPM have been developed according to the EMS-98 scale in order to have a specific correlation between

the mean damage grade, μ_D , and the probability to achieve a given damage threshold, D_K (Fig. 12) [12, 29].

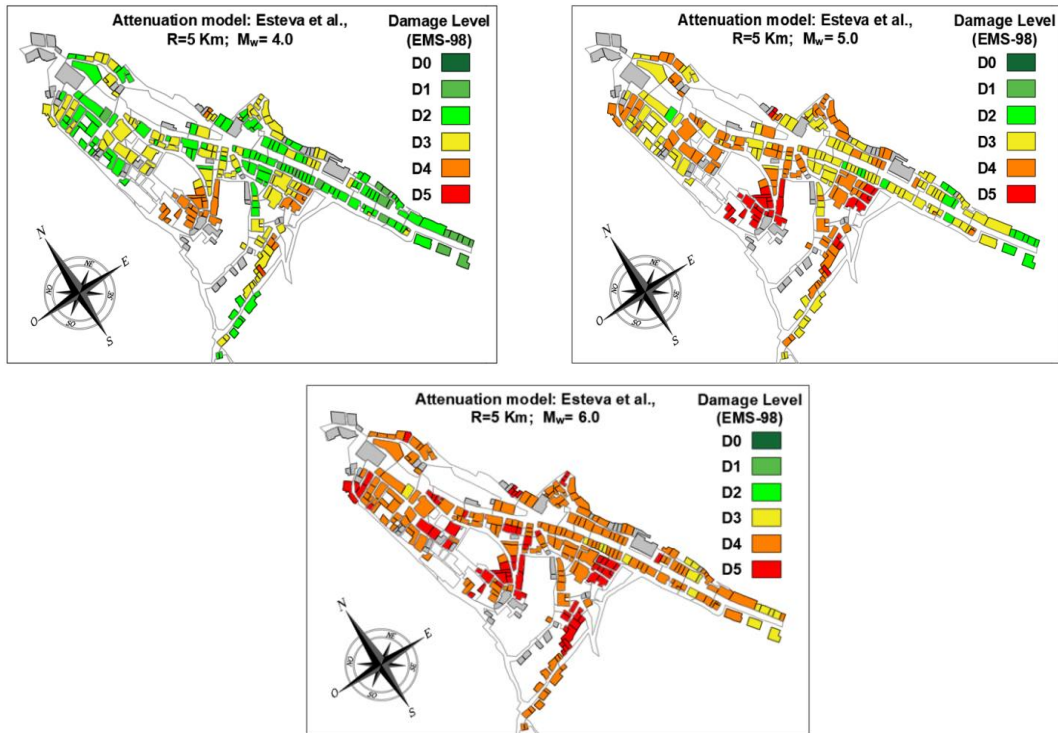


Fig. (11). Damage scenarios detected in the investigated urban area (for $R=5$ Km and M_w variable from 4 to 6) according to Esteva et al.'s attenuation law.

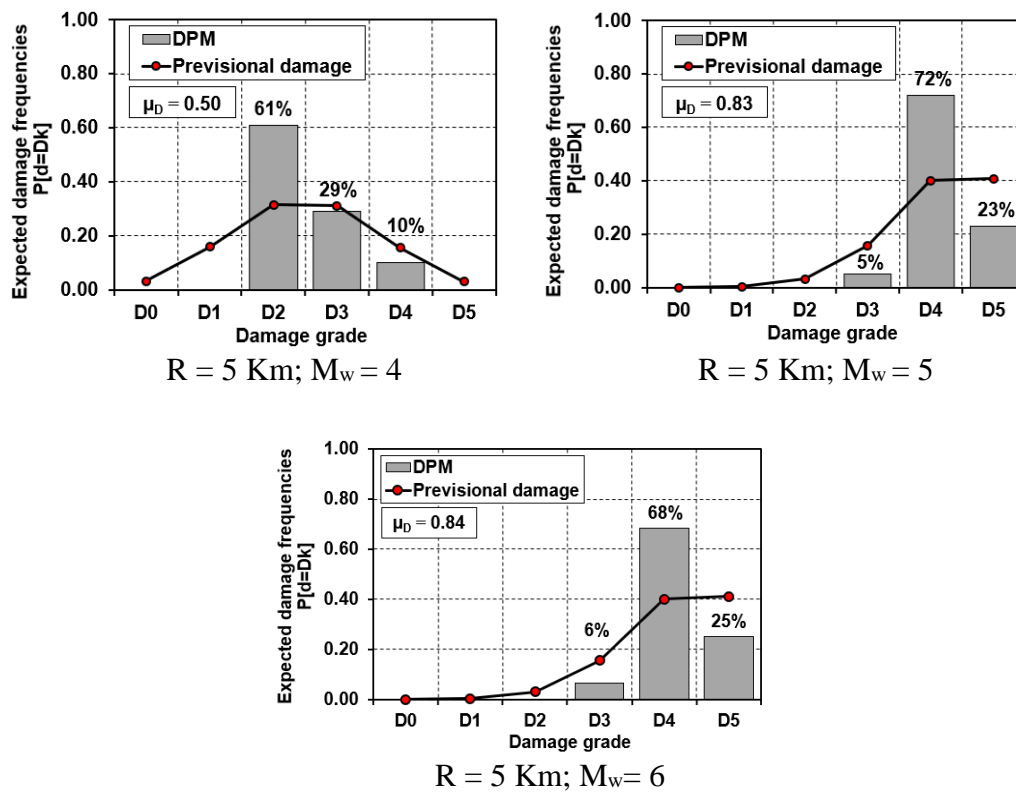


Fig. (12). Damage Probability Matrices for the selected seismic scenario.

The results have shown that for $M_w= 4$ about 61% of the building suffers damage D2, while for $M_w= 6$, damage thresholds D4 (near-collapse) and D5 (collapse) are attained in 68% and 25% of the cases, respectively. Contrary, when $M_w= 5$, a more variable damage distribution is achieved.

4. DISCUSSION

4.1. Vulnerability Analysis Considering Local Site Effects

4.1.1. Influence of the Local Hazard Conditions

In the framework of large-scale vulnerability analysis, the evaluation of local site effects represents one of the most important aspects to be considered for predicting the expected damage in a given area. In fact, when these effects are known, local authorities can implement suitable risk mitigation measures to safeguard people's life and preserve the integrity of historical heritage constructions.

From the physical point of view, the local seismic response can be intended as a set of changes in amplitude, duration and frequency content that a seismic motion, related to a bedrock basement, undergoes through the overlying layers of soil up to the surface. Generally, the main characteristics of the local hazard influence the seismic response of buildings. In particular, two important cases, such as the near-field and far-field earthquakes, could be considered. In the first case (*i.e.*, seismic events characterized by a reduced site-to-source distance, $R < 25$ km), the ground motion, near the seismogenic source, is associated with the vertically propagating compressive P-waves that, despite having a lower energy content than the horizontal ones (S-waves), tend to concentrate all the energy input in a restricted area, producing, in case of unfavourable site conditions, very devastating effects on the engineering structures. In the second case (far-field phenomena), the seismic waves effects tend to attenuate with the distance ($R > 100$ km) between the site and the seismic source [30 - 32].

As it is known, a generic ground motion can be described in either time or frequency domains. In the first case, the parameters most frequently used are the peak value of the acceleration, the velocity, the displacement and the duration. In the second case, the parameters of seismic motion are characterized by either Fourier spectrum or response spectrum. In the specific case study, the quantitative evaluation of the local seismic response, f_{PGA} , has been carried out based on the ratio between the maximum acceleration at the ground surface, considering the soil specific properties ($a_{max,s}$) and that at the bedrock ($a_{max,r}$), as reported in the following equation:

$$f_{PGA} = \frac{a_{max,s}}{a_{max,r}} \quad (6)$$

4.1.2. 1D-Simplified Approach for the Evaluation of Site Effects

The local amplification factor illustrated in Eq. (6) has been evaluated according to time domain based on the unscaled natural accelerogram of the event occurred in Molise in 2002

(epicentre located in Bonefro). The event was characterized by a magnitude, M_w , equal to 6.0 with a maximum $PGA= 0.55$ g. To simulate exhaustively local effects, STRATA 1.0 software [33], developed at the University of Texas, has been used considering a 1D numerical simulation of geological conditions of seismic motion at bedrock. Operatively, note the accelerogram at ground surfaces, it is possible to “relocate” the seismic input at the bedrock. In particular, once the soil stratigraphy is defined, the software implicitly takes into account the physical-mechanical characteristics of the soil layers.

So, based on this consideration, the soil profile has been assumed according to the study proposed in [12], characterized by a particular geological structure mainly derived from covering tectonics related to the formation of the Apennine chain and sub-Apennine reliefs with a bedrock basement located at 30 m of depth. In Fig. (13), the elaborated seismic motions with (amplified) and without (bedrock) local site effects have been plotted.

From the previous figure, the local site amplification factor, f_{PGA} , has been estimated as the maximum ratio between the detected accelerations (amplified and at bedrock) according to the aforementioned Eq. 6. As shown in Table 3, such a factor is equal to 1.33, which means that the seismic accelerations at the ground surface are 33% greater than those at the bedrock.

4.1.3. Influence of Site Effects on Vulnerability and Fragility Curves

Once the local amplification factor has been defined, it is possible to take into account the influence of the site effects in terms of typological vulnerability and fragility curves. Operationally, a more suitable vulnerability index of the inspected buildings, $V_{I,s}$, is calculated as the product between the contribution of the local amplification factor and the normalized vulnerability index:

$$V_{I,s} = f_{PGA} \times V_I \quad (7)$$

Therefore, it is possible to consider in a simplified manner the influence of the effects on the vulnerability assessment of the examined area. As shown in Table 3, the increase of vulnerability is directly proportional to the local amplification factor, estimated equal to 33%. Thus, the average typological vulnerability index, evaluated considering the site effects (Eq. 7), becomes equal to 0.73 and 0.48 for building classes A and B, respectively. Consequently, it seems quite evident that the increase of normalized vulnerability index has generated an almost proportional increase of the expected damage in terms of both vulnerability and fragility curves. Thus, the mean typological vulnerability curves, obtained by means of Eq. 4 (Section 3.1), have also been derived taking into account site effects, as shown in Fig. (14).

Furthermore, a synthetic representation of the new achieved damages has been done by means of DPMs shown in Fig. (15), where the previous combinations of magnitudes and site-source distances have been taken into consideration [12].

Table 3. Local amplification factor, f_{PGA} , for the Molise seismic event.

Time History	Acceleration [g]		
	Amplified - $a_{max,s}$	Bedrock - $a_{max,r}$	f_{PGA}
10 s	0.56	0.42	1.33

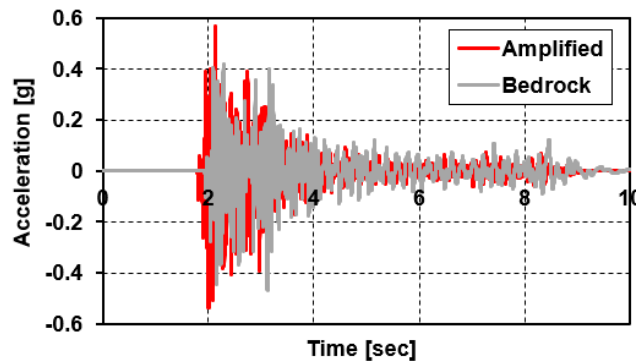


Fig. (13). The simulated accelerograms.

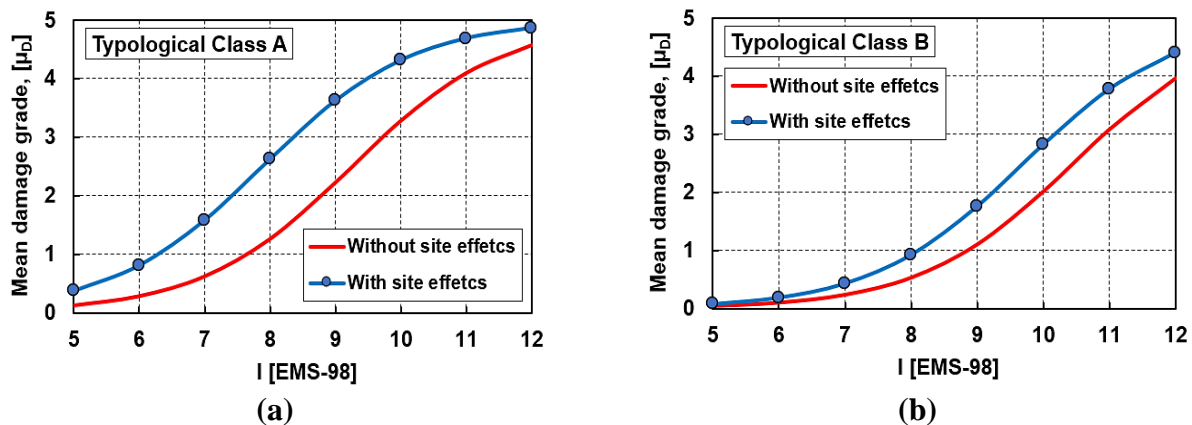


Fig. (14). Typical mean vulnerability curves considering local site effects for (a) class A and (b) class B buildings.

Thus, by comparing the new damage scenarios Fig. (15) with those reported in Section 3.2 (Fig. 12), it has been possible to evaluate the damage increase due to local site effects (Fig. 16).

The results have shown that, for an epicentre distance of 5 km, the damage distribution tends to increase towards higher damage levels as the magnitude increases. This result is markedly evident when considering $M_w=6$, since the damage level D4 is drastically reduced, while the D5 one is strongly increased.

Subsequently, the typical fragility curves related to the building classes examined considering the influence of soil conditions have been suitably elaborated and compared to those where geological effects have been neglected.

Therefore, as proposed in [34], the fragility curves have been derived considering the correlation law between macroseismic intensity and seismic acceleration:

$$\log(a_g) = C_1 \times I_{EMS-98} - C_2 \quad [g] \quad (8)$$

where the coefficients C_1 and C_2 have been assumed equal to 0.602 and 7.073, respectively. The comparison among fragility curves achieved with and without site effects is presented in Fig. (17).

The results show a clear preponderance of damage when site effects are considered for both typological classes examined. In particular, it has been observed that considering site effects, the damage probability tends to increase much more for ultimate limit states (from D3 to D5) than for serviceability limit states (D1 and D2).

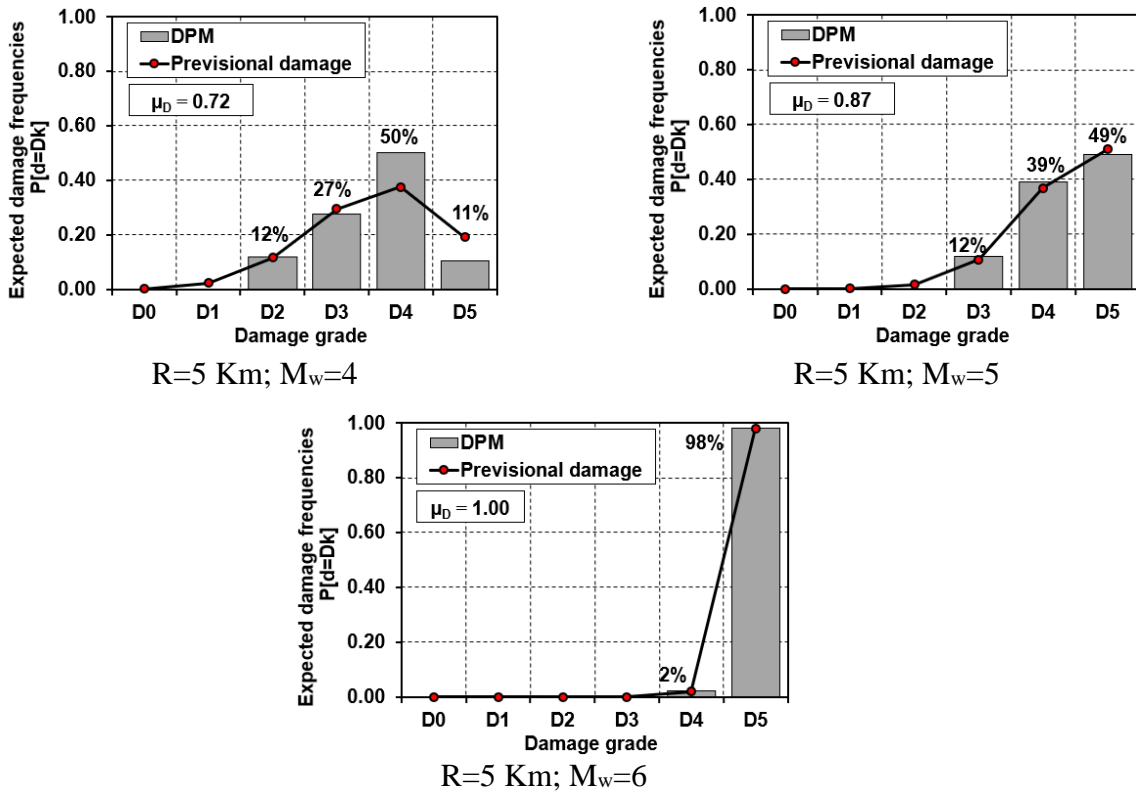


Fig. (15). DPMs considering local seismic amplification effects.

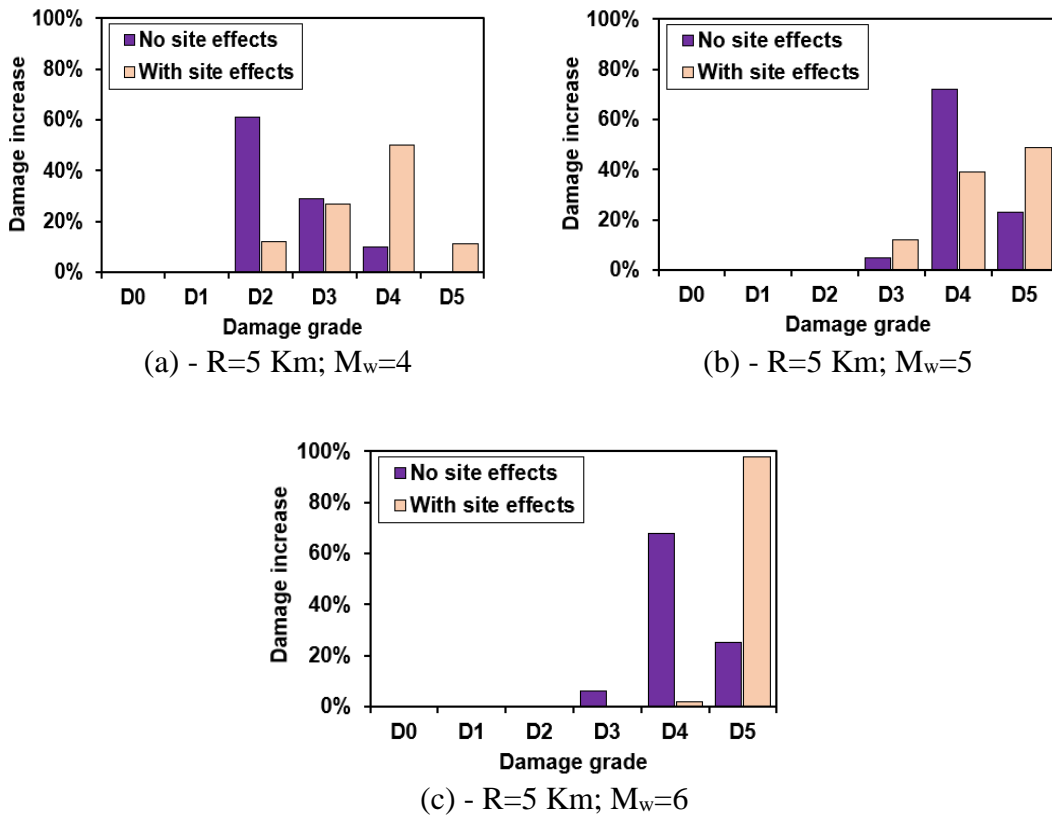


Fig. (16). Comparison among damage scenarios with and without site effects.

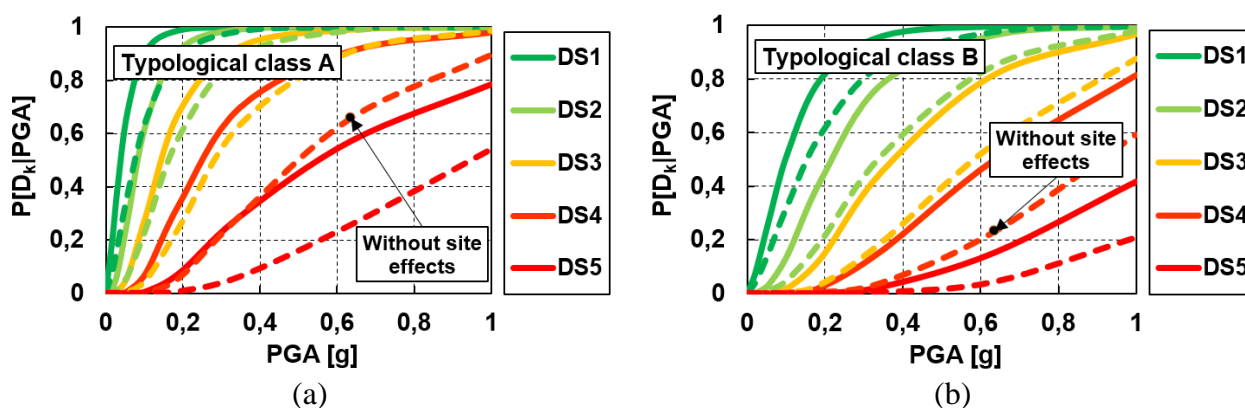


Fig. (17). Fragility assessment of the building classes with and without site effects for the typological classes analysed.

CONCLUSION

The study herein presented and discussed has proposed a novel and refined methodology to analyse the seismic vulnerability of masonry building aggregates in historic centres considering the influence of geo-hazard conditions. To this purpose, an urban sector with 300 buildings of the historical centre of Baranello in the Molise Region of Italy has been identified as a reference study area. The surveyed buildings have been firstly classified typologically and structurally according to the CARTIS form and then sub-divided in two different typological classes (A and B) based on the classification proposed by the European Macroseismic Scale EMS-98.

As a first step of the study, the seismic vulnerability assessment has been conducted by means of an empirical approach appropriately conceived for structural units of masonry building compounds.

Subsequently, for the quantification of the observed damage, the AeDES form, conjunctly with the Da.D.O database, have been adopted to statistically derive the Damage Probability Matrices.

Finally, the damage scenarios for different moment magnitudes and site-source distances based on the historical seismicity of the area have been derived by means of an appropriate seismic attenuation law. From the performed analyses, the following outcomes have been achieved:

- The vulnerability distribution for buildings of the typological class A is quite homogeneous, with an expected medium-high vulnerability level enclosed in the range [0.4-0.6]. Contrary, for buildings of the typological class B, the expected vulnerability index is in the range [0.2-0.4], which corresponds to a moderate vulnerability level.
- The results achieved from Damage Probability Matrices have shown that 38% of the buildings have damage D0 (null), while 36% of them reached a damage level D1 (negligible to slight damage).
- The damage scenarios of the investigated urban sector based on the Esteva *et al.*'s seismic attenuation

have provided, in case of moment magnitude, M_w , equal to 6.0, a damage threshold D4 (near-collapse) in 68% of buildings and a damage threshold D5 (collapse) in 25% of cases.

In the second study step, the site effects have been evaluated according to the simplified procedure based on the 1D half-space geological condition approach. To this purpose, the unscaled natural accelerogram of the event occurred in Molise in 2002 (epicentre located in Bonefro) with M_w of 6.0 and maximum PGA= 0.55 g has been considered. Local site effects have been taken into account through a local amplification factor given by the ratio between the maximum acceleration deriving from the specific soil type and that at the bedrock. From analyses carried out, the following results have been obtained:

- The stratigraphy of the site has produced an acceleration percentage increment of 33% compared to the bedrock acceleration;
- The global vulnerability of inspected buildings has been therefore increased due to local site effects. In particular, the average vulnerability index of typological classes of buildings A and B has become 0.73 and 0.48, respectively;
- The mean typological vulnerability curves have been modified considering geo-hazard conditions. In the worst analysis scenario, obtained for $R= 5$ km and $M_w= 6$, the site effects have drastically reduced the damage levels D3 and D4 and have increased significantly the threshold D5, with the collapse of 98% of buildings in the analysed area;
- The typological fragility functions of buildings have been derived considering local site effects. The results have shown a clear damage increase with site effects for both typological classes examined. In particular, it has been noticed that the damage probability tends to increase much more for ultimate limit states than for serviceability limit ones.

In conclusion, the proposed work has provided a simplified effective approach for evaluating site effects in large-scale

analysis of masonry building compounds. Such an analysis method could represent an important tool for predicting damage scenarios in historic centres in a more precise way. Further developments of the study can foresee the application of the methodology to other seismic regions characterized by different types of masonry buildings and geological conditions to plan more effective seismic risk mitigation interventions.

CONSENT FOR PUBLICATION

Not applicable.

AVAILABILITY OF DATA AND MATERIALS

The data that support the findings of this study are available from the corresponding author, [A.F], upon reasonable request.

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None.

CONFLICT OF INTEREST

The authors declare no conflict of interest, financial or otherwise.

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